

Strength Characteristics of GGBS and Steel Slag based Binary Mix Concrete

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Abstract

The objective of this study is to experimentally study the effect of partial replacement of Portland cement and fine aggregate by the industrial wastes ground granulated blast furnace slag (GGBS) and steel slag (SS) respectively, on the various strength parameters of concrete. Totally 9 mixes were proposed according to ACI standards, with varying replacements of cement with 40%, 50% and 60% of GGBS and varying replacement of fine aggregate with steel slag by 10%, 20% and 30% weight of concrete. The compressive strength using cubes of size 100 mm x 100 mm x 100 mm and splitting tensile strength using cylinder of size 100 mm x 200 mm were found out for curing periods of 14 and 28 days respectively for all the mixes. Results were then compared with conventional concrete and the optimum replacement percentage of GGBS and steel slag is reported.

Keywords: Ground granulated blast furnace slag; Steel slag; Compressive strength; Tensile strength.

1. Introduction

Research on alternate binders and supplementary cementitious materials has been going on for many years. The cement production is a highly energy intensive process which consumes a high amount of energy. For the manufacture of Portland cement, each ton of its produce releases 1 ton of carbon dioxide approximately [1]. The production of cement contributes to around 5% of the global greenhouse gas emissions [2]. Use of industrial by-products as replacement to the cement will reduce the quantity of cement used in construction. This will result in reducing the energy for the cement manufacture thereby reducing the greenhouse gases emissions. The use of the industrial by-product GGBS in concrete, which would contribute to land pollution, will reduce the greenhouse gas emissions from construction industry.

The use of ground granulated blast furnace slag (GGBS) and steel furnace slag (SFS) in construction has been in practice from a long time, going as far back as a century in the United States and about 150 years in Europe [3]. The possible use of steel slag with benefits related to technological and ecological aspects were studied from researches [4, 5].

Partial replacement of cement with high percentages of ground granulated blast furnace slag was tested for compressive strength and flexural strength. The optimum amount of replacement was found out to be 55% [6]. The optimum replacement percentage for steel slag in place of fine aggregate was found to be 40% [7] while using unprocessed steel slag increased strength up to 30% of replacement [8].

1.1 Research Significance

The main objective of this research is to find the optimum percentage of combined partial replacement of cement with the industrial waste ground granulated blast furnace slag (GGBS) and

the partial replacement of fine aggregate with SS with respect to the compressive strength and the splitting strength of the slag based concrete. This will help reduce the usage of cement without forsaking the strength of the concrete. Also the requirement of huge amounts of sand can be tackled with the help of steel slag replacing sand.

2. Materials and Methods

2.1 Materials Used

The materials used to study the characteristic strength of concrete constitute of cement, GGBS, steel slag, M-sand, 12.5 mm aggregates. Cement utilized for the study was ordinary Portland cement of 43 grade confirming [9]. Ground granulated blast furnace slag having a specific gravity of 2.6 was used to partially replace cement in various percentages as shown in table 1. M-sand passing through a sieve of 4.75 mm having a specific gravity of 2.81 was used as fine aggregate. Steel slag of specific gravity 2.8 which was having a fineness modulus of 2.78 was used for partial replacement of fine aggregate and the following mix proportions were proposed (Table 1).

2.2 Methods

Mix proportion according to ACI standards is proposed for a target strength of M60 [10]. High binder content of 504.21 kg/m³ is casted with M-sand as fine aggregate of 683.24 kg/m³ and coarse aggregate of size 12.5 mm of 1108.1 kg/m³. The fine aggregate used with particle size confirmed to the requirements of ASTM C33 [11]. The following mix proportion table gives the content of cement, GGBS, M-sand, steel slag and coarse aggregate. Cement was then replaced with varying percentages of GGBS as 40%, 50%, and 60% by weight of cement whereas M-sand was replaced with steel slag in different percentages of 10%, 20%, and 30%. For example, the mix G4S1 is a mix with cement

replaced with 40% of GGBS by weight of cement, fine aggregate M-sand replaced with 10% of steel slag by weight of fine aggregate. In this respect, a total of 9 mixes were proposed. A water-cement ratio of 0.29 is taken and the water content is 141.6 kg/m³. Polycarboxylate is used as super plasticizer and is added as 1.7% to the weight of cement content.

Table 1: Mix Proportion (kg/m³)

Specimen	Cement	GGBS	M-sand	Steel Slag	Coarse Aggregate
Control	504.21	0	683.2	0	1108.1
G4S1	302.5	201.7	614.9	68.3	1108.1
G5S1	252.1	252.1	614.9	68.3	1108.1
G6S1	201.7	302.5	614.9	68.3	1108.1
G4S2	302.5	201.7	546.6	136.6	1108.1
G5S2	252.1	252.1	546.6	136.6	1108.1
G6S2	201.7	302.5	546.6	136.6	1108.1
G4S3	302.5	201.7	478.3	205	1108.1
G5S3	252.1	252.1	478.3	205	1108.1
G6S3	201.7	302.5	478.3	205	1108.1

2.3 Compressive Strength

The compressive strength of the binary mix was measured with the help of cube compressive strength of 100 mm x 100 mm x 100 mm cubes confirming to the code [12]. The cube specimens were casted in the order of the proportions as shown in table 1. Required amount of GGBS was taken and mixed with cement to form the binder for the concrete. These were further mixed with steel slag and M-sand. Coarse aggregates of 12.5 mm were added to the mixture and sufficient water, including super plasticizer is poured into the mix to get the concrete. This is then molded into cubes of dimensions, 100 mm x 100 mm x 100 mm. After 24 hrs. the cubes were de molded and kept under curing for curing periods of 14 days and for 28 days. After 14 days and 28 days, the cubes were taken out of curing phase and kept in the open air for about 1 day and tested for strength.

2.4 Splitting Tensile Strength

The splitting tensile strength of concrete was measured using 100 mm length x 200 mm diameter cylinders [13]. The cylinders were casted in plastic PVC pipes which were molded to the requirement. The cylinders were carefully de molded and kept for the same number of curing period as of cubes, i.e., 14 days and 28 days. 9 mixes were casted and after curing for 14 and 28 days, they were tested under ACTM.

3. Results and Discussions

In this section, the results obtained from the ACTM are reported in tables (tables 2, 3) and are comprehensively compared with each other as well as with control concrete. The trend of increase or decrease in strength is discussed as follows.

3.1 Compressive Strength

The compressive strength was conducted in the automatic compression testing machine (ACTM) at a loading rate of 2.9 kN/s. (refer fig. 1). The results are published in table 2. A comparative analysis reported as follows.



Fig. 1: Compressive strength test setup

Table 2: Compressive strength (MPa)

Specimen	14 days strength	28 days strength
Control	31.01	50.65
G4S1	20.91	28.63
G5S1	35.03	40.3
G6S1	37.46	39.2
G4S2	21.01	25.34
G5S2	36.74	40.87
G6S2	40.49	48.87
G4S3	23.58	37.26
G5S3	37.98	47.34
G6S3	44.11	54.78

The compressive strength reduces when compared to control concrete but the strength increases as the GGBS amount in the concrete increases. The percentage increase itself decreases with the increase in the quantity of GGBS. For example the compressive strength of G4S1 is 20.91MPa for 14 days while that of control concrete is 31.01 MPa. There is a clear reduction in strength. As the GGBS quantity is increased from 40% by weight of concrete to 50% by weight of concrete, the strength increases to 35.03 MPa. The percentage increase is 67.53%, whereas the percentage increase from 50% replacement to 60% replacement of GGBS is 6.93% only. From this we can infer that the effect of GGBS in the strength decreases as its quantity increased further 60% replacement by weight.

Contrary to the GGBS addition, the addition of steel slag increased the compressive strength as can be seen from the percentage increase with 10% fine aggregate replacement and 20% fine aggregate replacement.

Together, addition of GGBS and steel slag gives good strength. The percentage increase in strength for G6S3 compared to control concrete is 8.15%.

Mixes with low steel slag and low GGBS gives low strength and the optimum mix with respect to compressive strength is G6S3.

3.2 Splitting Tensile Strength

The cylinders were tested in ACTM as shown in fig. 2.



Fig. 2: Splitting tensile strength test setup

Table 3: Splitting Tensile Strength (MPa)

Specimen	14 days strength	28 days strength
Control	1.8	2.7
G4S1	2.63	2.69
G5S1	2.47	2.96
G6S1	2.87	3.05
G4S2	2.70	2.85

G5S2	2.52	2.69
G6S2	2.94	2.73
G4S3	2.76	2.84
G5S3	2.77	2.79
G6S3	3.02	3.16

The splitting tensile strength of GGBS, steel slag based concrete increased with respect to control concrete. With the increase in the content of steel slag, the splitting tensile strength of specimens increased and this can be seen in the research [8]. The mixes with 50% GGBS had a reduction in tensile strength but it was more than that of control concrete. The best mix with good tensile strength is G6S3.

4. Conclusions

1. Compressive strength decreased for the concrete with composite binder compared to conventional concrete whereas splitting tensile strength increased.
2. As the quantity of GGBS and steel slag increased, compressive strength and splitting tensile strength increased.
3. The percentage increase in strength decrease with further addition of GGBS.
4. The optimum mix is found to be concrete with 60% GGBS and 30% steel slag and further increase do not increase the strength further.
5. Addition of steel slag may cause corrosion and the corrosive properties of the mixes can be checked in further studies which would yield better durability of concrete.

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